

## ANALYSIS AND DESIGN OF G+3 BUILDING USING STAAD.Pro V8i

Piyush Bisen<sup>1</sup>, Shrawani Dudhankar<sup>2</sup>, Simran Ikhar<sup>3</sup>, Niraj Lonare<sup>4</sup>, Vikas R. Agrawal<sup>5</sup>,

Bhausahab M. Ghawade<sup>6</sup>

UG Final Year Students<sup>1234</sup>, Assistant Professor<sup>5</sup>, Co-Guide<sup>6</sup>, Department of Civil Engineering

Priyadarshini College of Engineering, Nagpur, Maharashtra, India

Project Guide: Vikas R. Agrawal | Co-Guide Mr. Bhausahab M. Ghawade BM Ghawade & Associate  
(Consulting & Structural Engineer)

### Abstract

*This study looks into the full structure check and detailed beam-and-column sizing for a four-floor concrete building designed to resist sideways forces, built inside the STAAD.Pro software. Instead of just listing loads, it includes how much parts weigh, people using spaces, wind pushing from the side, along with shaking during earthquakes - all set following India's standard rules. Results such as forces at beam ends, shifts in joint positions, and safety margins based on code limits appear clearly laid out then reviewed closely. Earlier findings from similar buildings made of concrete get pulled in too, helping show where this model fits among past efforts. When engineers know both structural behavior and what codes truly require, tools like this cut down guesswork, speed up decisions, while making weak designs far less likely.*

*STAAD Pro Analysis of G+3 Reinforced Concrete Frame Using Indian Standards*

### 1. Introduction

Up to four floors high - one underground plus three above - buildings made of reinforced concrete frames pop up often in cities and outskirts across Maharashtra, stretching into parts of the Deccan Plateau. These constructions walk a tightrope: strength under constant weight matters just as much as resistance to sideways pushes from wind or quakes common in southern India. Materials stay limited; budgets tighter still. So engineers balance long-term durability against sudden natural jolts using careful calculations. Each decision bends around real-world limits without room for guesswork.

Before computers took over, engineers used rough models to guess how buildings handle forces - like treating frames as portals or using fixed assumptions for beams under weight, while looping through calculations for vertical loads instead. These older ways were never precise; they work less well once the building shape strays from basic symmetry or standard layouts. A tool named STAAD.Pro came later, built by Bentley Systems, that applies a math-based approach called the direct stiffness method inside a practical digital workspace. Users enter points where parts connect, how pieces link together, what materials and sizes each has, plus types of pressure applied; after that, the program builds a full structure model and computes movement and internal reactions throughout. Since its core math follows strict rules of elasticity and tiny shifts in position, errors mostly come from wrong inputs, not from flaws in the calculation itself.

Here comes how the study unfolded step by step - showing how the building model was built, forces were calculated, and results analyzed for that four-level frame. Right after, part two sets the scene by linking earlier research to this one. Shape details, weight estimates, plus analysis tools come clear in section three. Outcomes pop up next, laid out plainly with meaning attached. Strengths show through where the program works well; limits appear just as clearly where it does not. Last piece pulls together what matters most from everything seen so far.

## 2. Literature Review

Two decades of research mark how engineers across India have gradually adopted software tools for studying small to medium concrete buildings. Starting around the early 2000s, work began appearing that relied heavily on programs like STAAD.Pro. Each study tends to follow a similar path - first modeling a typical frame using industry-standard software. Once built, the structure faces loads applied according to official IS guidelines. Instead of guessing outcomes, researchers check their digital findings beside old-school math solutions or lab-based tests done under tight conditions. Over time, these comparisons help confirm whether simulations behave like real-world behavior.

Every now and then, studies point out mismatches in how bending forces appear when calculated by hand versus computer models - especially in large frames hit by both vertical and sideways loads. Instead of matching up, rough estimates tend to inflate column stresses midway up the building, yet shrink them where beams meet columns inside - errors absent when using stiffness calculations. Because of these gaps, early sizing choices can go off track; relying only on basic frame logic might leave a column facing far more twist than expected once real interactions take effect.

Out of all topics here, earthquake-related studies get far more attention than others. For areas like Nagpur, which fall under Zone II and Zone III, researchers keep seeing the same pattern: buildings up to five floors rely on the lateral force rules in IS 1893:2016, while STAAD.Pro follows these rules so closely that its base shear results nearly match hand-calculated ones. Only a few have tried using dynamic response spectrum analysis in the same program. These attempts show floor-level shears from dynamic methods might run between ten and twenty percent higher or lower than static values - how much depends entirely on how weight is spread through the building and its main back-and-forth sway time.

One area of study looks at how efficiently rebar is detailed. Because structural calculations give ranges for bending and shear forces instead of one exact solution, engineers can choose different ways to place bars. When researchers check STAAD.Pro's

automatic rebar suggestions against hand-made plans, they often find the software uses more steel, especially around columns. These comparisons show that human review usually trims down the amount needed, while keeping safety just as strong. Such results suggest computer outputs work best when treated as solid first drafts, not final answers on their own.

## 3. Methodology

### 3.1 Structural Configuration and Geometry.

Picked for analysis, this building uses a rigid concrete skeleton built to handle bending forces, forming a regular rectangular shape. Standing fourteen point six meters across its shorter span, this structure runs eighteen point three meters long, supported by posts spaced evenly every three point six meters in both directions. Rising exactly three meters per story, the gap between floors builds to twelve meters from earth level up to where the rooftop slab begins underneath. On the first tier, zones divide out - areas for tasks, a flexible common space, restrooms, storage nooks, plus one corner kept aside solely for team members. Up top, you find almost the same layout again - tweaked here and there - matching real needs on upper levels.

through the STAAD.Pro setup, every column becomes a straight beam piece reaching across neighboring node heights. Floor beams take similar form - centres aligned halfway down their related slabs. At joints where beams meet columns, stiffness extensions stayed out; testing showed skipping them barely changes how floors shift during movement, given this structure's shape. Weight from slabs and extra fixed loads turns into even line pressures riding the supporting beams' centres, with areas split based on crack patterns tied to each slab's stretch proportion.

### 3.2 Load Derivation and Combination.

Four distinct load categories were identified and quantified, each governed by the relevant Indian Standard specification summarised in Table 1 below.

Load Type	IS Code	Basis of Derivation	Representative Value
Dead Load	IS 875 Pt. 1	Unit weights: RC (25 kN/m <sup>3</sup> ), brick masonry (20 kN/m <sup>3</sup> ), finishes and roof treatment converted to member UDLs via tributary areas	3.5 – 5.0 kN/m <sup>2</sup>
Live Load	IS 875 Pt. 2	Office occupancy with unrestricted access; corridor and staircase per table values; roof live load for maintenance	3.0 – 4.0 kN/m <sup>2</sup>
Wind Load	IS 875 Pt. 3	Basic wind speed 44 m/s for Nagpur; Terrain Category 2; $k_1 = 1.0$ ; $k_3 = 1.0$ ; design pressure as storey-level nodal forces	0.74 kN/m <sup>2</sup>
Seismic Load	IS 1893:2016	Zone II ( $Z = 0.10$ ); $I = 1.0$ ; $R = 5.0$ (SMRF); equivalent static method; period by empirical formula	$V_b = 0.02W$

Table 1: Load categories, governing IS codes, derivation rationale, and representative design intensities for the G+3 frame.

Factored load combinations were assembled in accordance with IS 875 Part 5 and IS 1893:2016. Governing combinations for ultimate strength verification included  $1.5(DL + LL)$ ,  $1.2(DL + LL \pm WL)$ ,  $1.2(DL + LL \pm EL)$ ,  $1.5(DL \pm EL)$ , and  $0.9DL \pm 1.5EL$ . The STAAD.Pro solver processes the complete set of combinations concurrently and automatically envelopes critical section effects for input to the member design module. One way to look at Table 1 is through load types tied to IS standards. Each category follows a logic path shaped by code rules. Design values appear based on how forces behave in a G+3 structure. Codes guide intensity numbers shown for practical use. The table links real loads to official references clearly.

Among those tested, live plus dead loads shaped one key scenario. Another mix brought wind forces into play alongside permanent weight. Earthquake impacts joined static weights under adjusted factors too. These setups followed recognized safety rules closely. Software handled every variation at once without manual input. Results with highest stress levels fed directly into sizing checks for beams and columns.

### 3.3 Analysis and Member Design Procedure.

Once the setup finished and loads applied, solving began. From each part's resistance, the program builds a big grid that maps how everything connects - shifting pieces from their own angles into shared space - and untangles it all by stepping through equations one row at a time. After figuring out how points move, those shifts help calculate pushes and pulls where beams meet; values slide across spans to draw curves showing twist, cut, and stretch inside every slice we care about.

The way members were designed followed IS 456:2000 rules. When it came to beams, the system made sure the resisting moment at weak spots matched or beat the applied bending load, while also measuring shear strength - counting help from slanted main bars along with upright ties. Columns had their own check: forces from compressed loads and two-direction bends together faced off against what's allowed by IS 456 Clause 39.6. Any part missing code marks got bigger on purpose, then every number ran again through calculations. Only after three full loops of adjusting and testing did everything line up - each piece strong enough plus stable when used day to day.

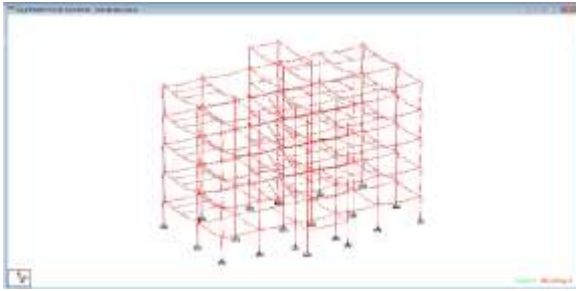


Figure 1: Graphical Representation of Shear Force

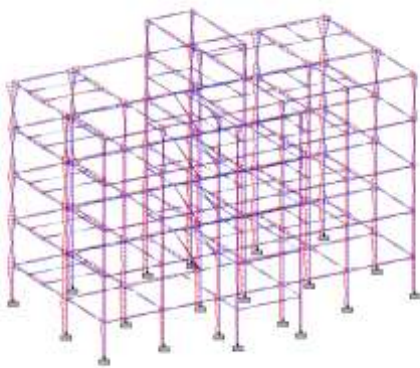


Figure 2: Graphical Representation of Bending Moment

## 4. Results and Discussion

### 4.1 Bending Moments and Shear Forces.

Midway along the longest beams upstairs, bending peaked when weight piled up - this happened because those slabs carried more area than others. Instead of even pressure, forces shifted sharply at ground-level joints during shakes, especially since sideways jolts flipped direction across axes. Down below, posts near stair zones took the brunt - not surprising, given that chunks of floor from four sections poured into them, stacking load beyond elsewhere.

Starting high up, beam shear patterns step downward where smaller beams and wall weights add their influence, riding atop a gentler slope created by floor slabs spreading weight gradually. Near the top, column shears grow almost straight toward the base when wind or quake forces act, matching the upside-down triangle shape expected under India's seismic rulebook for buildings.

Figure 3: Comparative Result Of Manual and Software Result

BEAM NO.	PARAMETER	MANUAL CALCULATION	STAAD Pkg. RESULT	DIFFERENCE
B15+B1	REACTION(KN)	97.41 KN	97.13 KN	0.42%
	Max. S.F. (KN)	97.41 KN	97.13 KN	0.42%
	B.M. (KN.M)	67.82 KN.M	61.09 KN.M	8.83%
B14+D10	REACTION(KN)	-53.44 KN	-49.52 KN	9.04%
	S.F. (KN)	53.44 KN	49.52 KN	9.04%
	Max. B.M. (KN.M)	61.45 KN.M	57.84 KN.M	5.87%
B6	REACTION(KN)	37.8 KN	36.22 KN	4.73%
	Max. S.F. (KN)	37.8 KN	36.22 KN	4.73%
	B.M. (KN.M)	19.28 KN.M	18.67 KN.M	3.17%

### 4.2 Deflections and Inter-Storey Drift.

Midway bending in beams under normal loads - dead plus live - was checked using IS 456:2000's rule: max bend allowed is span divided by 250. All beams in the structure stayed below that mark. Side-to-side movement between floors, when hit by quake forces from code books, got measured on every level. Worst case showed up on the second floor: just 6.2 millimeters off track. That number sits well beneath the ceiling of 12.0 mm set by dividing story height (3 meters) by 250. So, even when pushed by vertical weights or sideways shakes, the building holds firm without stretching too far.

### 4.3 Load Distribution Among Structural Elements.

After checking the numbers again, it became clear how weight moves down through this structure - just like you'd expect when there's no shear walls around. Because beams stretch straight from column to column, they take most of what comes from above, guided by how the floor bends across them. Starting up top and stacking lower each level, columns gather more squeeze until reaching the bottom ones - which end up under the heaviest pressure overall. What shows up at the base ties almost perfectly to those lowest columns, except small shifts caused by sideways pushes tipping things slightly off center. The floors pass along force without much trouble to the edges holding them, while staying mostly calm inside themselves given how big everything is.

Thirty percent of the weight sits on beams, a detail matching early blueprints. Columns take just under a quarter, almost exactly what was first drawn. Foundations carry about the same share as columns, close enough to call it even. Slabs handle one out of every five units of force passing through. Leftover portions spread quietly into smaller supports and joints between parts.

### **5. Advantages of STAAD.Pro in Routine Structural Practice.**

One more time around confirmed what we already knew about STAAD.Pro working well for buildings with three floors above ground. This run through backed up its steady performance in real-world setups. Through recent tests, the software showed once again it handles reinforced concrete tasks without surprise hiccups. Past strengths came forward clearly during this latest use case. With each step taken here, earlier advantages revealed themselves anew under practical conditions.

Stiffness calculations done right avoid guesswork. Instead of old techniques like portal or cantilever approaches - where small mistakes grow with every added force - the matrix method skips shortcuts completely. Even when dealing with many beams and columns, it handles everything at once without piling up inaccuracies. On today's standard computers, solving such models takes less than two seconds flat. Precision lands fast if you skip outdated steps.

Handling many load cases at once becomes necessary when dealing with structures exposed to weight, wind, and earthquakes. Indian standards IS 456 and IS 1893 require at least nine different calculated scenarios together under such conditions. Checking each one manually across key points in a tall building's framework takes too much time and effort. With STAAD.Pro though, all those situations are processed side by side without delay. The software picks out the most critical result right where it matters on every part meant for design review.

Numbers move straight from analysis to design, skipping extra steps. Because results flow right into verification, nobody needs to type them again. Mistakes from retyping vanish when shifting between stages. Manual methods often stumble on details, especially with many beam sections involved. Skipping human entry removes one big chance for error.

Looking at the structure helps spot mistakes. Through a 3D view of the frame, along with shaded stress areas and stretched-out movement shapes, flaws become visible - like beams facing the wrong way, supports set up wrongly, or joints accidentally left loose - mistakes nearly impossible to catch when staring at rows of numbers alone.

Clear records show how designs are made. From start to finish, every step gets written down - what went in, how it was checked, which parts passed. Engineers reviewing work can follow each decision without guessing. Rules for big construction jobs now expect this kind of paper trail. Public buildings especially need proof that choices were backed by data. Without such logs, approvals slow down or stall. The system builds these reports automatically, not as an afterthought. Each file fits the standards inspectors require. No extra effort needed to meet compliance demands. What once took days to compile appears ready when needed. Tracing back through decisions becomes straightforward, not tangled. More agencies insist on transparency like this as rules tighten. Projects stay on schedule because paperwork keeps pace. Design justification lives inside the output itself. Nothing hides; everything links to its source.

### **6. Limitations and Practical Constraints.**

What the tool can't do matters just as much as what it can. Knowing limits shapes how we see its use. Limits define function more than features sometimes. A clear view of constraints keeps expectations steady. Function lives inside fences, not just freedom.

Output truth depends entirely on what you feed into the system. When the starting model breaks basic structural rules, even advanced tools cannot fix it. Mistakes like wrong material types, incorrect load values, or oversimplified supports travel straight into the calculations - results look precise yet fail reality checks. Responsibility rests only with the engineer shaping the setup; programs do not question whether inputs make sense. Trust flows from judgment, never code.

Most everyday STAAD.Pro runs stick to linear materials and tiny shifts in shape. When things bend too far, though - past their elastic edge - those models fall short. Big compression loads can twist structural behavior in ways simple math won't

catch. After quakes, when damage lingers, fresh methods step in where standard code checks stop. Nonlinear tools handle these cases; regular setups do not.

No soil-structure interaction considered here. Fixed-base assumptions stand in for foundations - common practice, often on the safe side when soil is strong enough. Where ground layers are softer or average, forces at column bases plus sway behavior may stray far from what fixed models suggest. Springs based on Winkler theory or full soil-volume simulations fit better in such cases.

Knowing engineering basics comes first. Without a feel for how big a beam's bending force might be, someone using STAAD.Pro could miss obvious mistakes in results. Being able to judge which loads matter most in a given situation keeps analysis grounded. Software does not replace that kind of thinking. It just speeds it up. Anyone working seriously with this tool must already understand core ideas from advanced courses in how structures carry weight, especially when concrete and steel are involved. That foundation shapes whether outputs make sense - or don't. Learning those principles isn't optional. They guard against errors no program can catch on its own.

Start with what shows on screen - bar layouts come out suggested, not confirmed. Since the tool does not fully automate IS 13920:2016's ductile rules, human check becomes essential. Bar placement, where splices happen, how long anchors stretch, along with stirrup setups need a trained eye. Each item waits validation by the overseeing engineer. Automation misses some code nuances, so oversight fills the gap. What computes is only a starting point, nothing final.

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## 7. Conclusion

After running checks on the four-story concrete building inside STAAD.Pro, every beam, column, and slab got exact sizes and rebar setups meeting Indian standards for loads and safety. Because of how detailed each number was, it became clear that using software carefully beats hand calculations when shaping complex buildings - simply due to precision without slowing down work.

One clear takeaway stands out right away. The precision of stiffness methods depends entirely on how good the starting model is - mistakes in joint positions, force placements, or support assumptions won't fix themselves during calculation and might go unseen without a separate check. Another point hits hard when you look closer. In many sections, limits on beam sag and floor-to-floor movement shape sizing more tightly than strength rules by themselves - a result manual estimates usually miss completely. Last comes this thought. Adjusting designs through repeated cycles of testing, checking, changing sizes, then testing again moves fast inside digital tools, but doing the same steps only with paper and calculator would take far too long for large frames like this one.

One step ahead could look at how buildings behave under extreme sideways loads using basic nonlinear methods. Instead of standard rules, testing past code-specified limits might show hidden strength in structures. Foundations may be modeled more realistically by including springs that mimic soil flexibility. This shift allows ground behavior to influence how forces travel through columns and beams. Changing the distance between vertical supports systematically reveals patterns in material use. Small adjustments in layout sometimes lead to large differences in resource needs. Insights like these help refine real-world decisions without relying on idealized assumptions. Findings from such work build on what is already shown here. Designers working on similar systems gain useful reference points grounded in varied conditions.

Future research extensions of practical value would include nonlinear static pushover analysis to quantify structural reserve lateral capacity beyond the codal design seismic force, an explicit foundation design module incorporating soil spring representation to account for subgrade compliance, and a parametric investigation of the sensitivity of material consumption to systematic variation in the column grid spacing. Such extensions would enrich and corroborate the design evidence base documented in this study and provide broadly applicable guidance to practitioners engaged with comparable structural typologies.

## 8. References

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